



**2022 SME Minnesota
Conference**

The Impact of Stress History on Undrained Shear Strength of Clays in Northern Minnesota

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Barr Engineering Co.
Minneapolis, MN

Agenda

- Geologic characteristics
- Stress history
- Drained vs. undrained behavior
- Link between stress history and shear strength
- Assessment methodology
- Results
- Hypothetical example

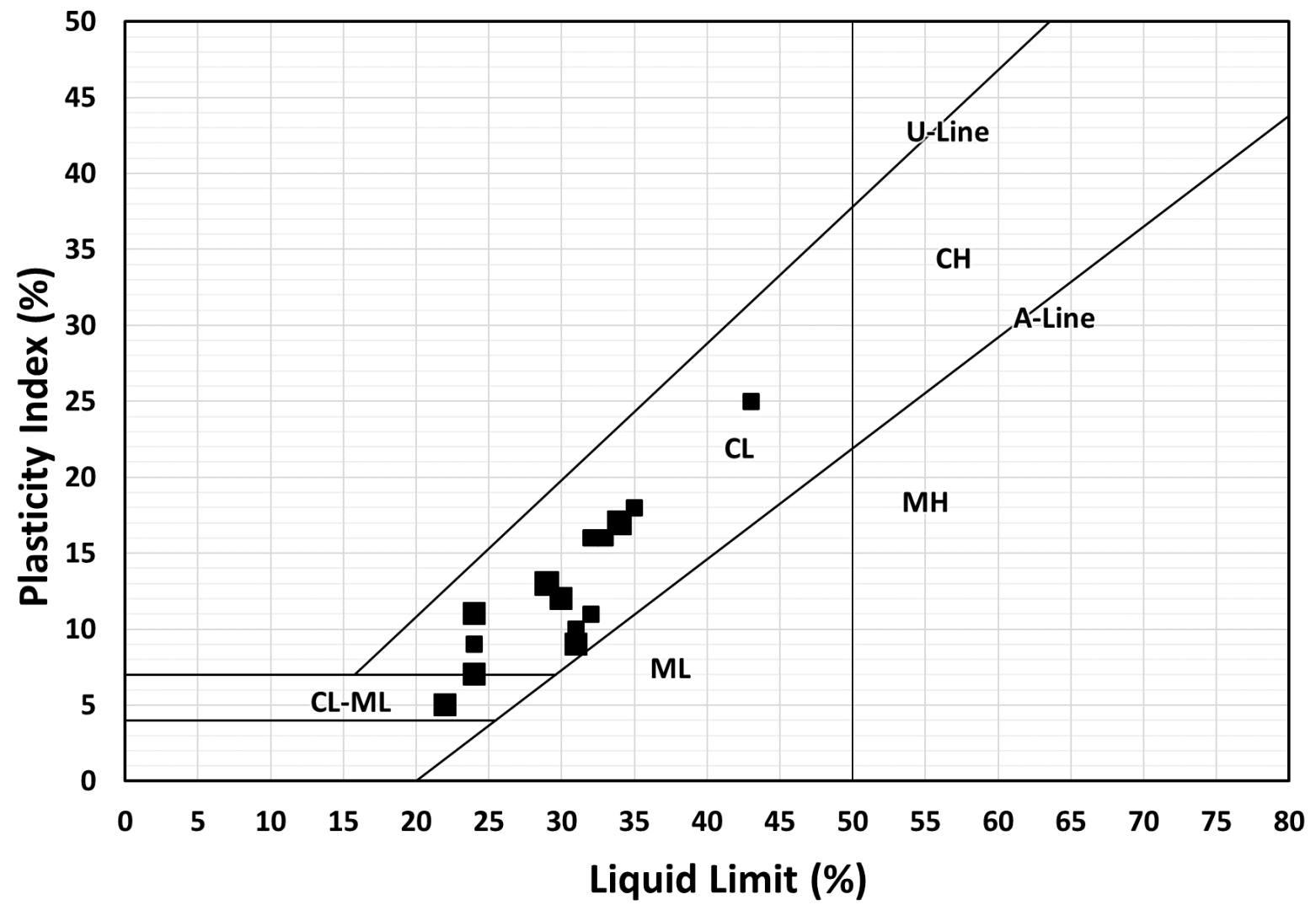
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Geologic Characteristics

- Widespread glacial deposits – clay and sands
- Glacial till - fine-grained soils with gravel, cobbles, and boulders
- Clays are generally over-consolidated and stiff to hard due to glaciation

Geologic Characteristics (cont.)



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Stress History

$$\textit{Over Consolidation Ratio} = \textit{OCR} = \frac{\sigma'_p}{\sigma'_{v0}}$$

Stress History (cont.)

$$\text{Over Consolidation Ratio} = OCR = \frac{\sigma'_p}{\sigma'_{v0}}$$

Max past pressure



σ'_p



Current pressure

Stress History (cont.)

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Max past pressure

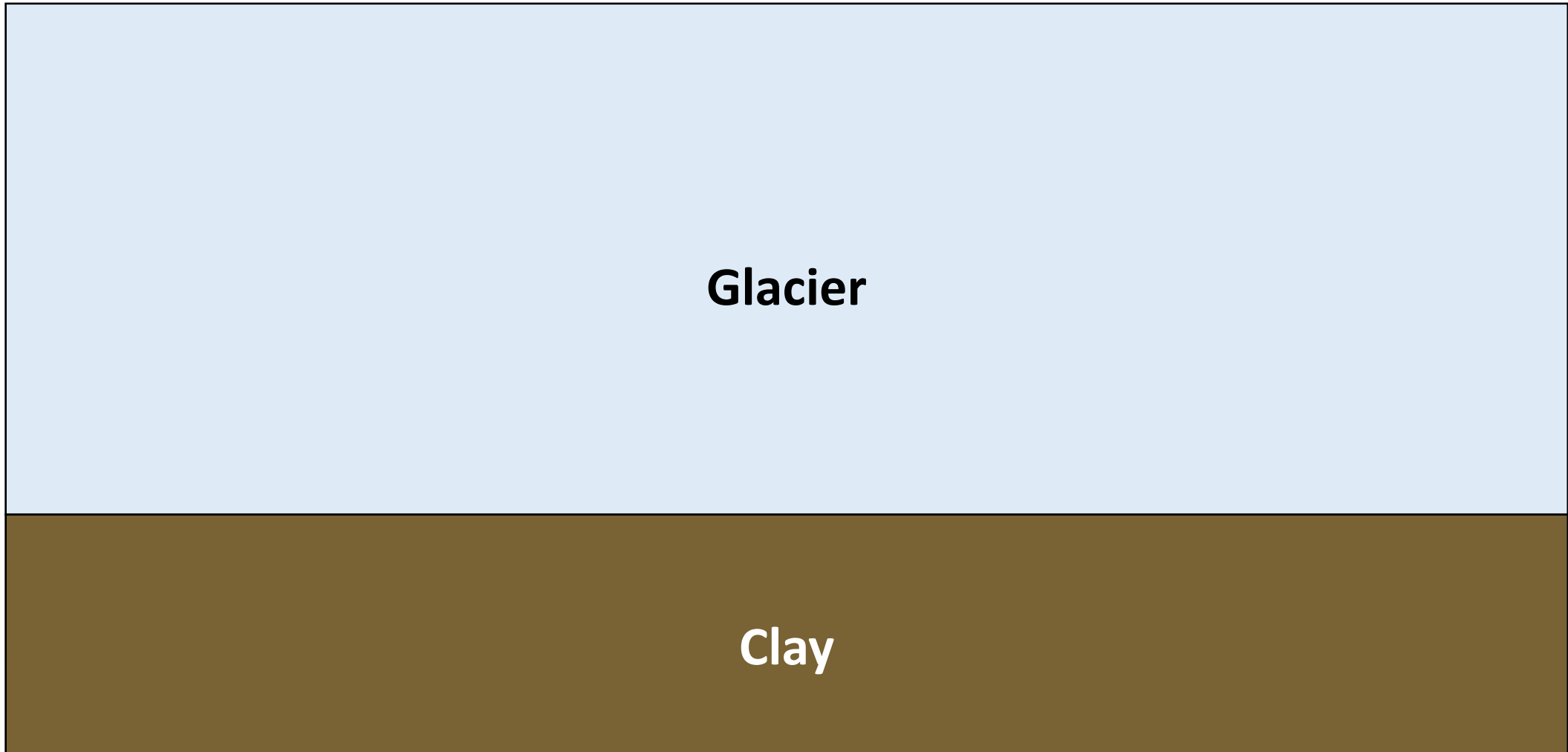
Current pressure

OCR	Condition	General Behavior
1	Normally Consolidated	Weak, compressible
1-3	Slightly Over-Consolidated	Stronger, less compressible
>3	Heavily Over-Consolidated	Strong, stiff

Stress History (cont.)

Clay

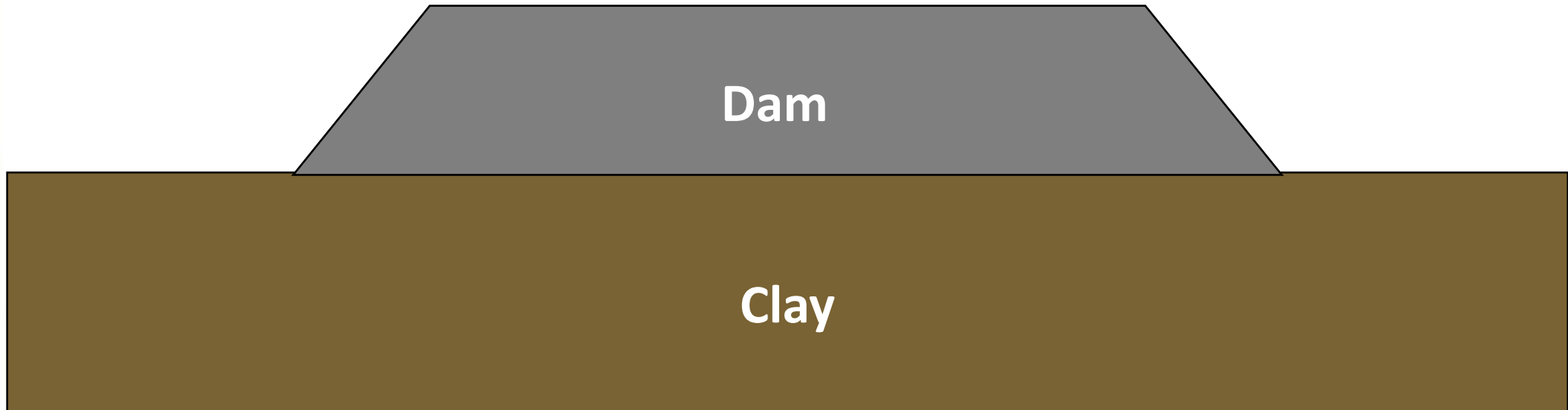
Stress History (cont.)



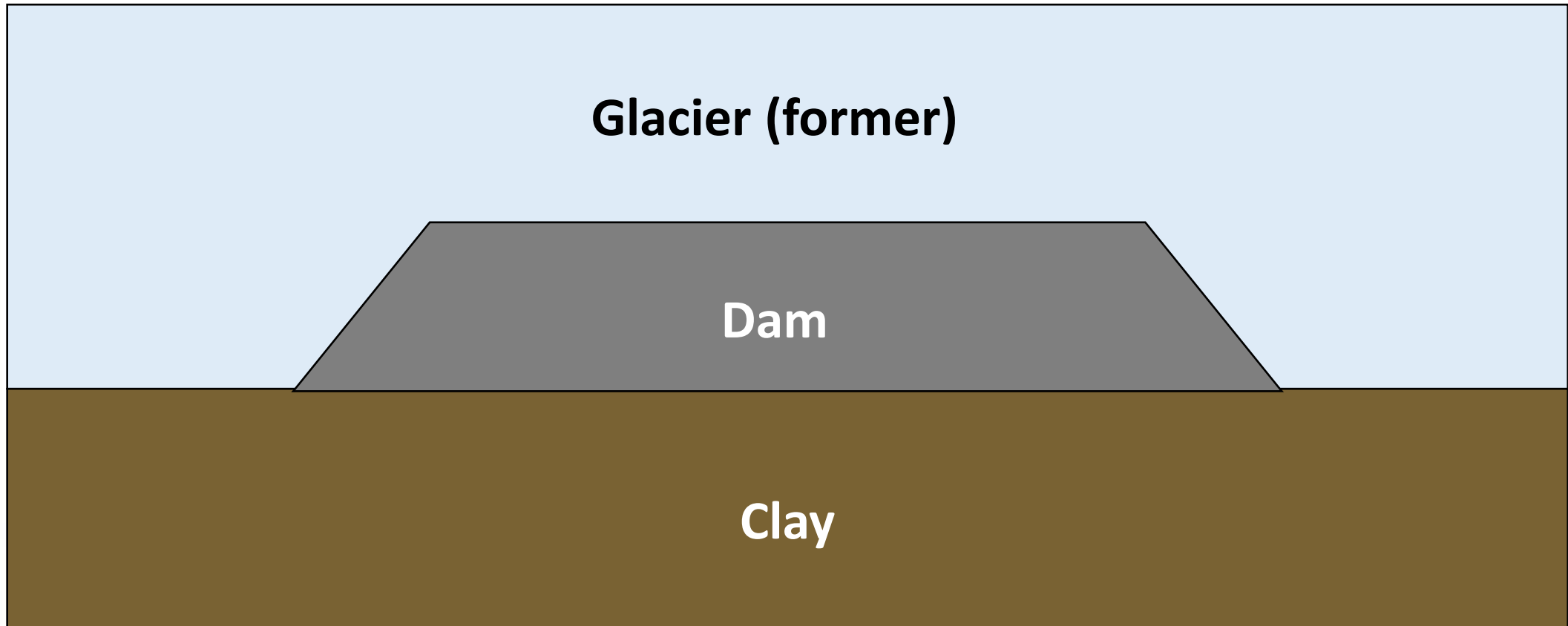
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Clay

Stress History (cont.)

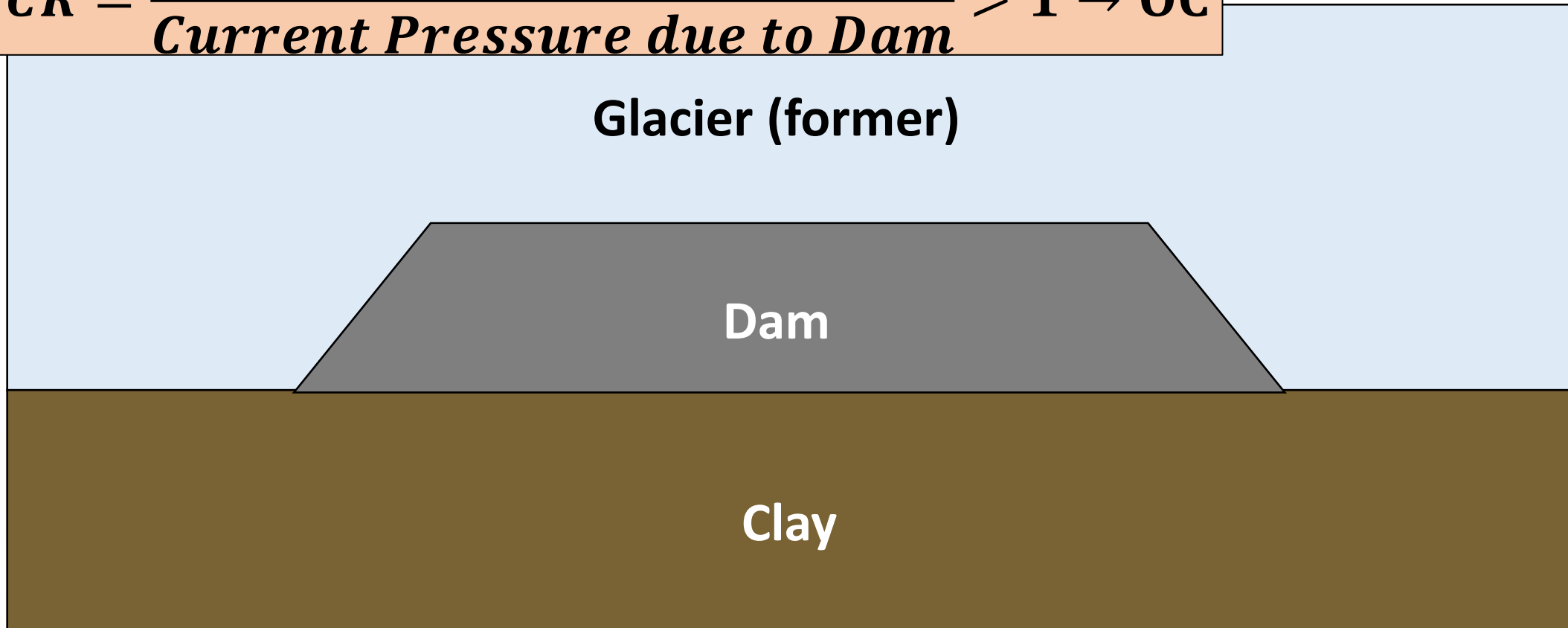


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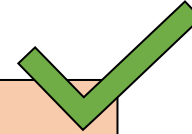
Stress History (cont.)

$$OCR = \frac{\text{Past Pressure due to Glacier}}{\text{Current Pressure due to Dam}} > 1 \rightarrow OC$$



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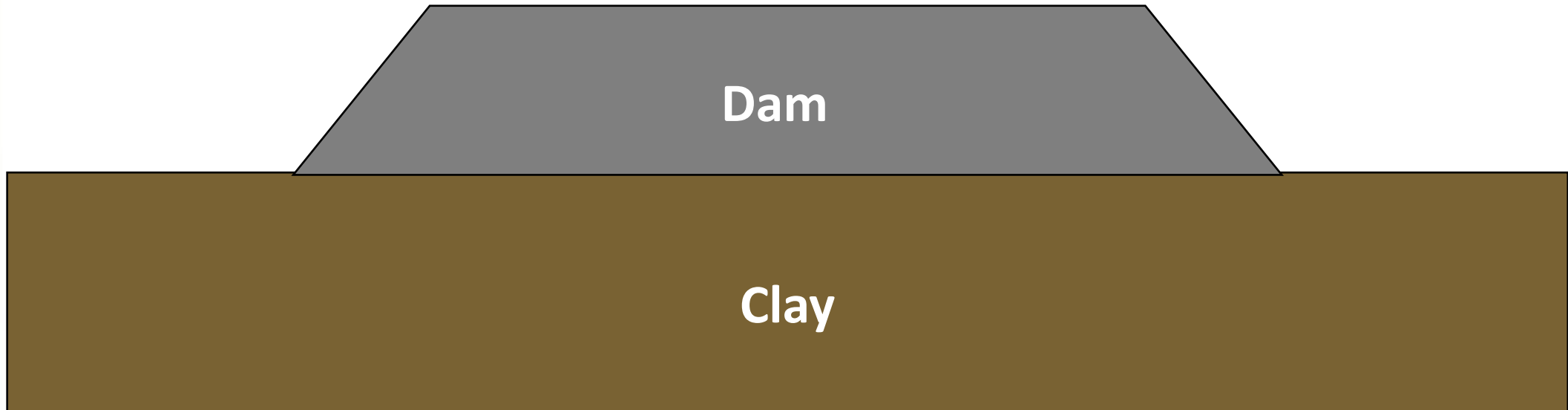


Glacier (former)

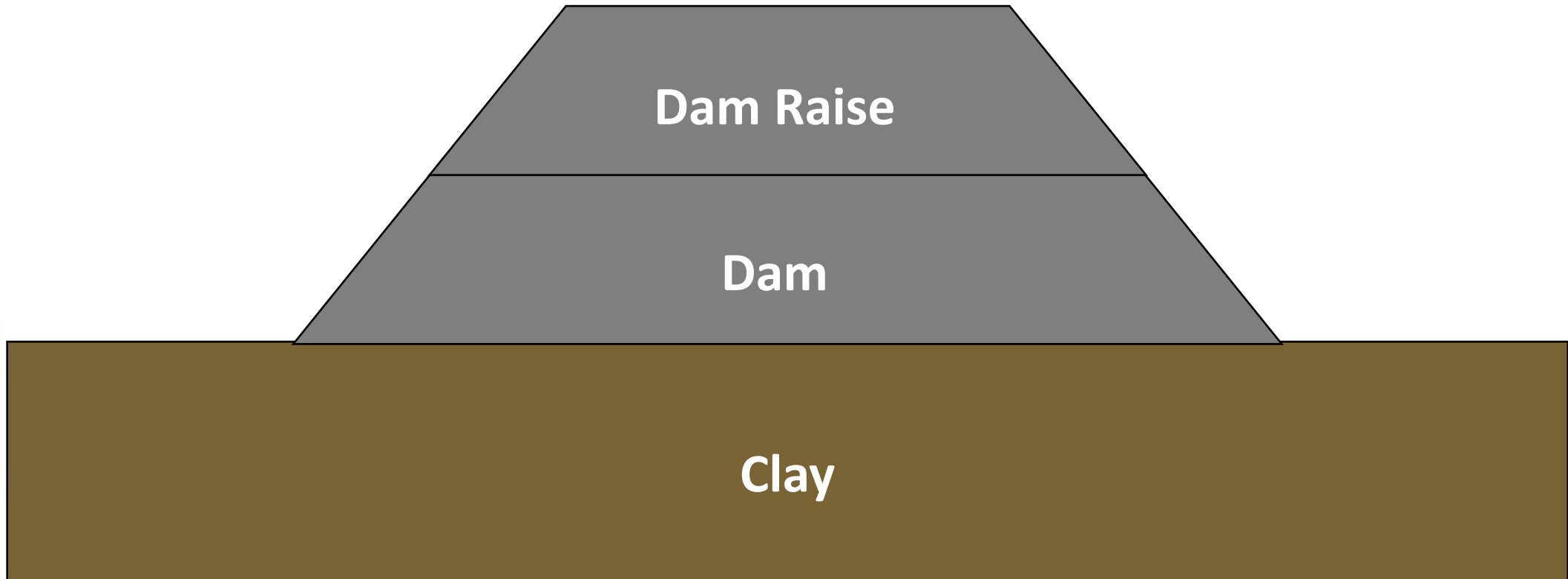
Dam

Clay

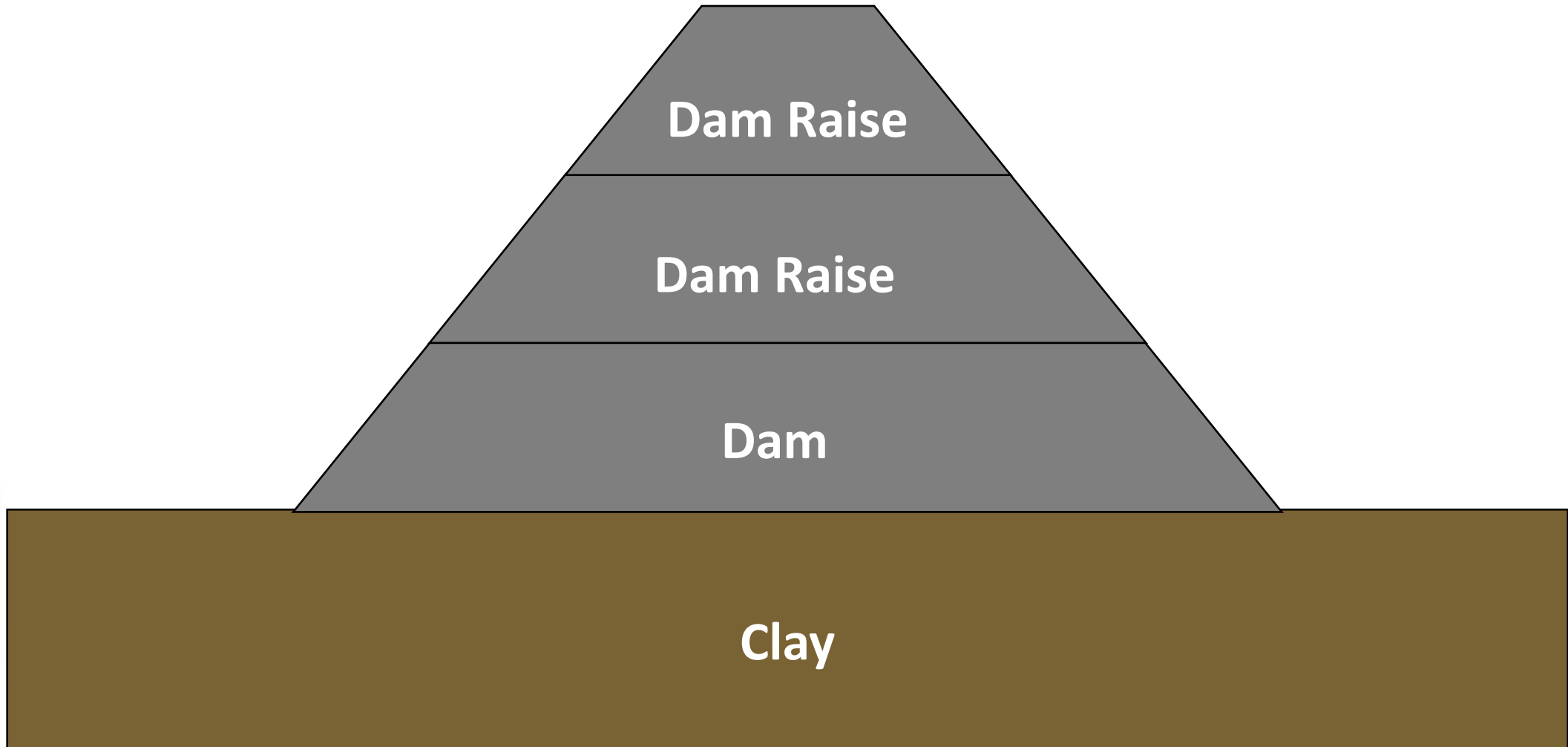
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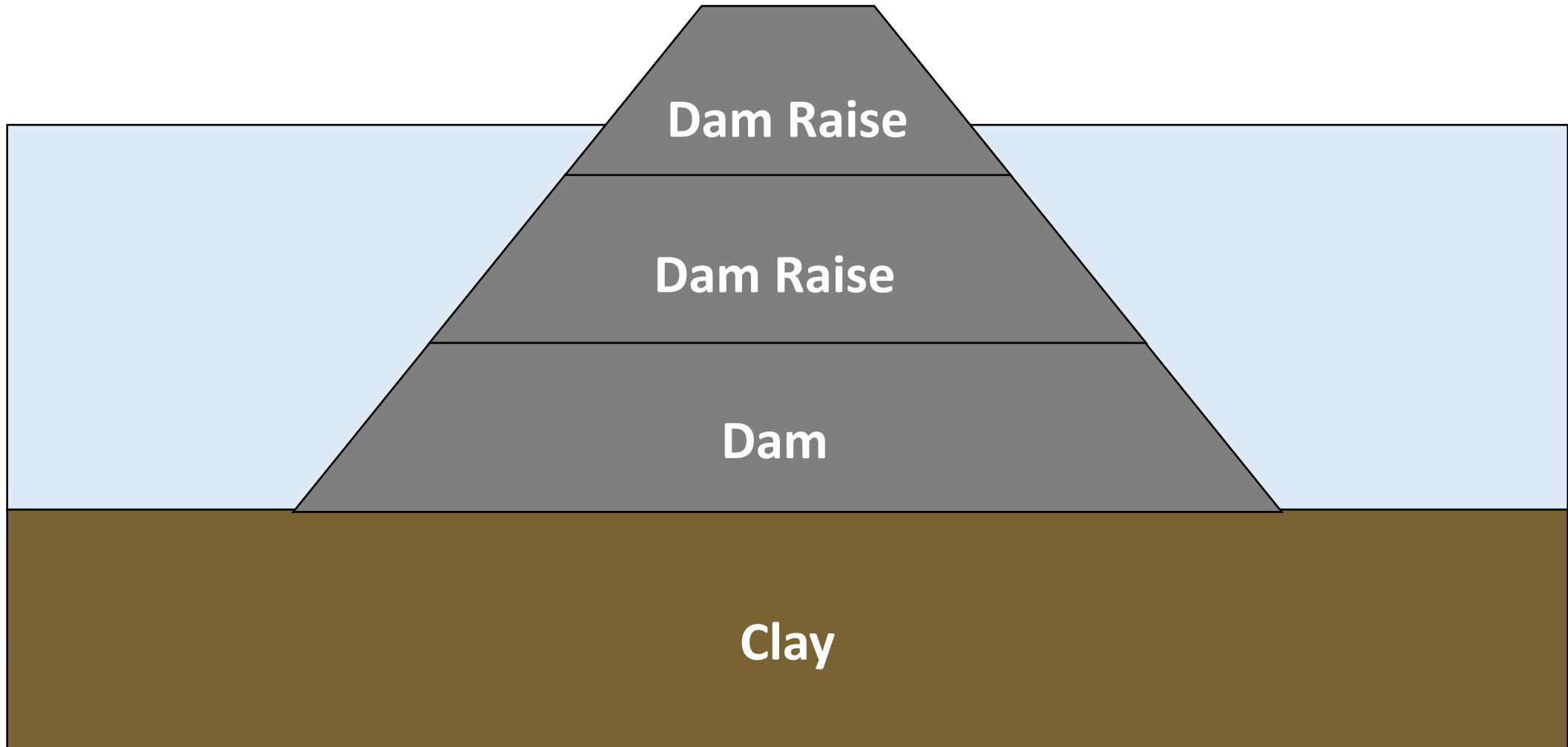
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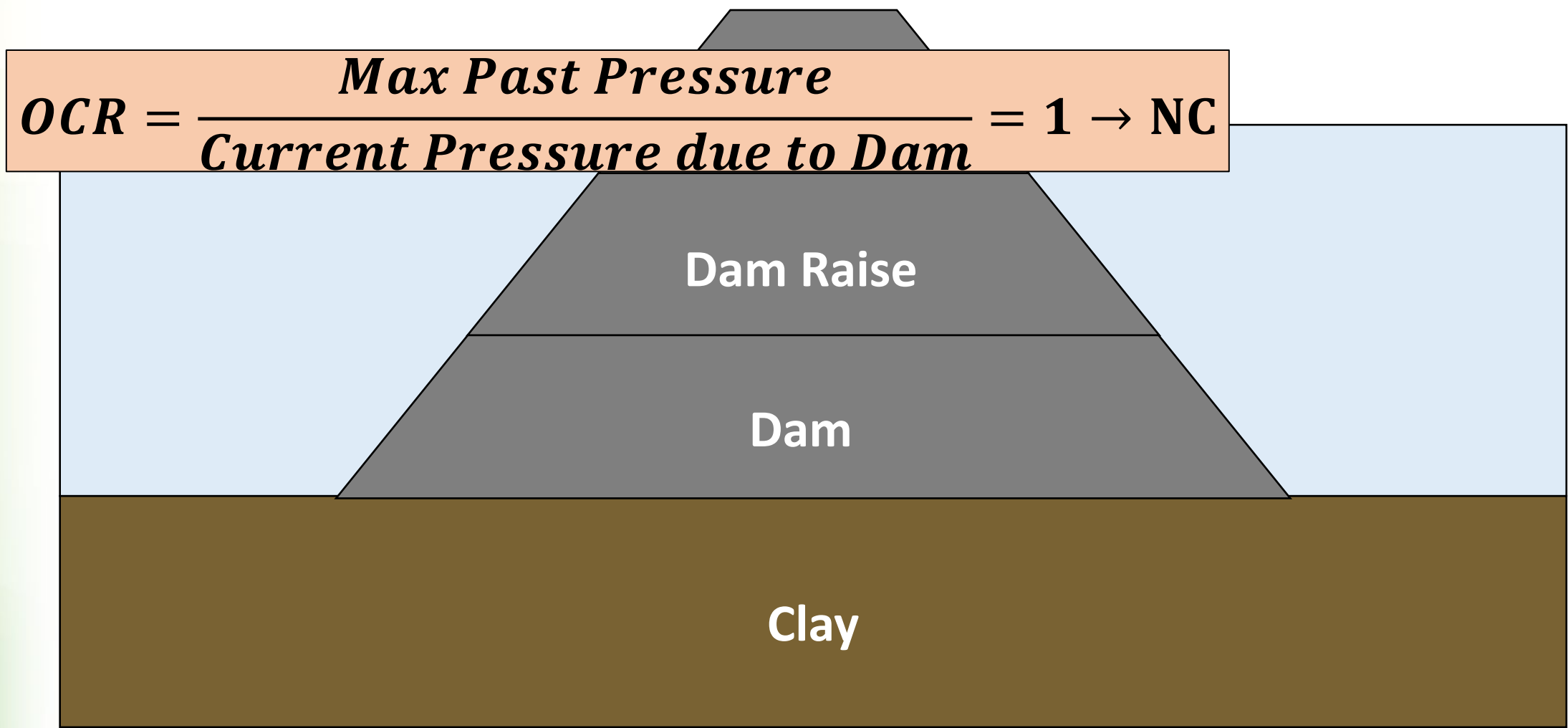
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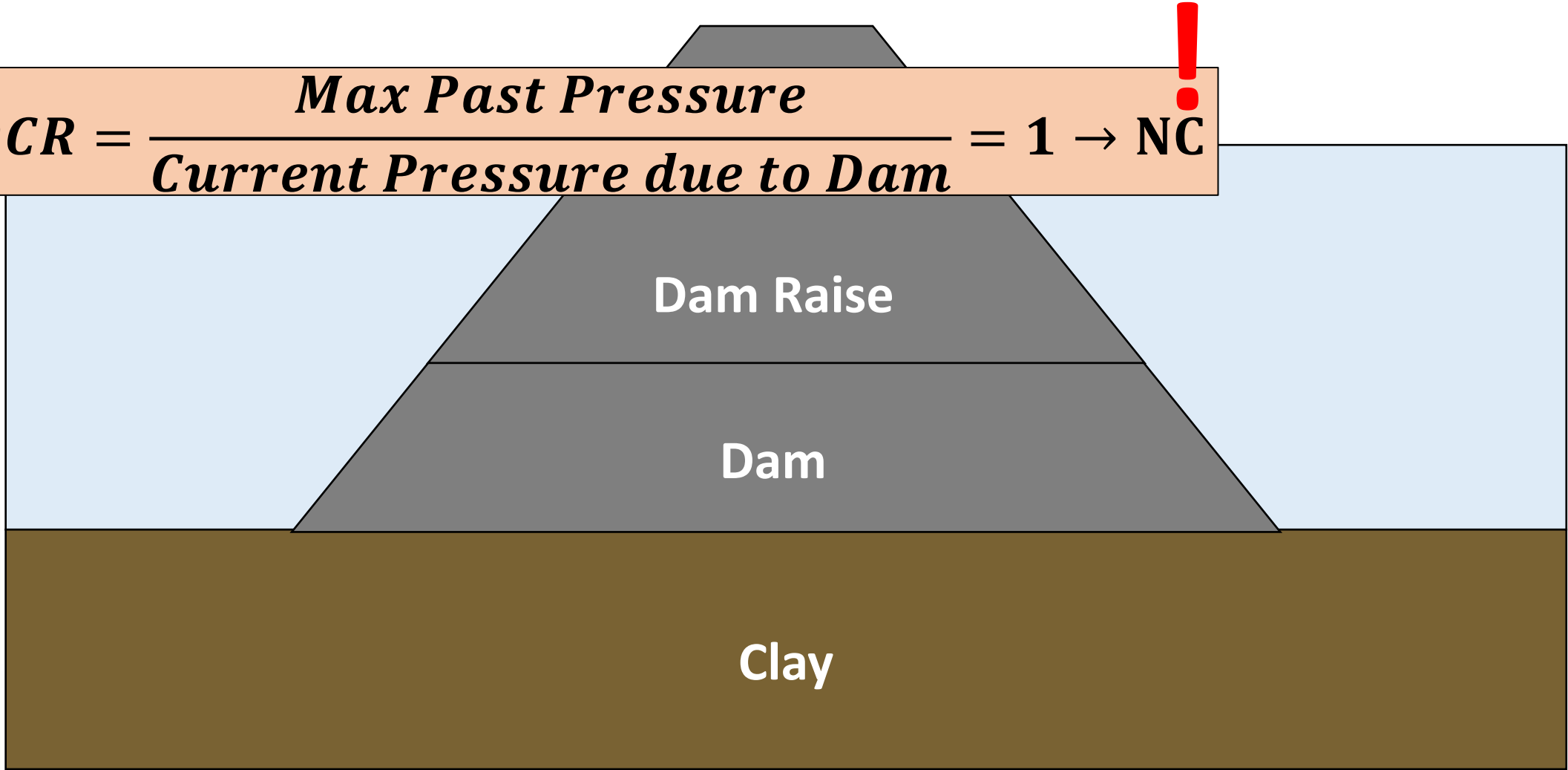


Stress History (cont.)



Stress History (cont.)

$$OCR = \frac{\text{Max Past Pressure}}{\text{Current Pressure due to Dam}} = 1 \rightarrow \text{NC}$$



Common Mistake...
assuming a stiff, over-consolidated
clay will remain that way

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- Geologic characteristics
- Stress history
- **Drained vs. undrained behavior**
- Link between stress history and shear strength
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Drained vs. Undrained Behavior

- Pertains to rate of shear/loading and material permeability

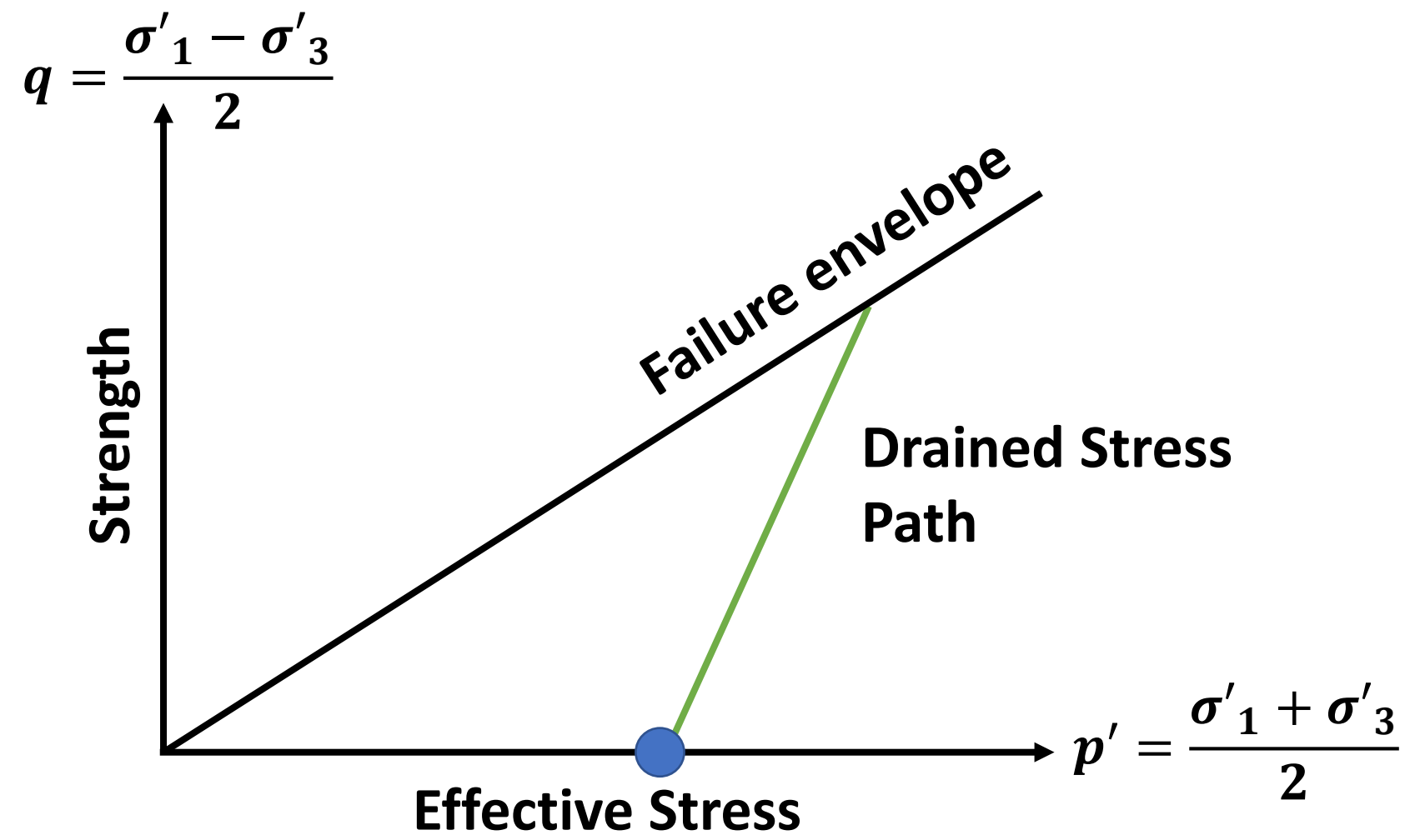
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- If excess pore-water pressures develop → undrained

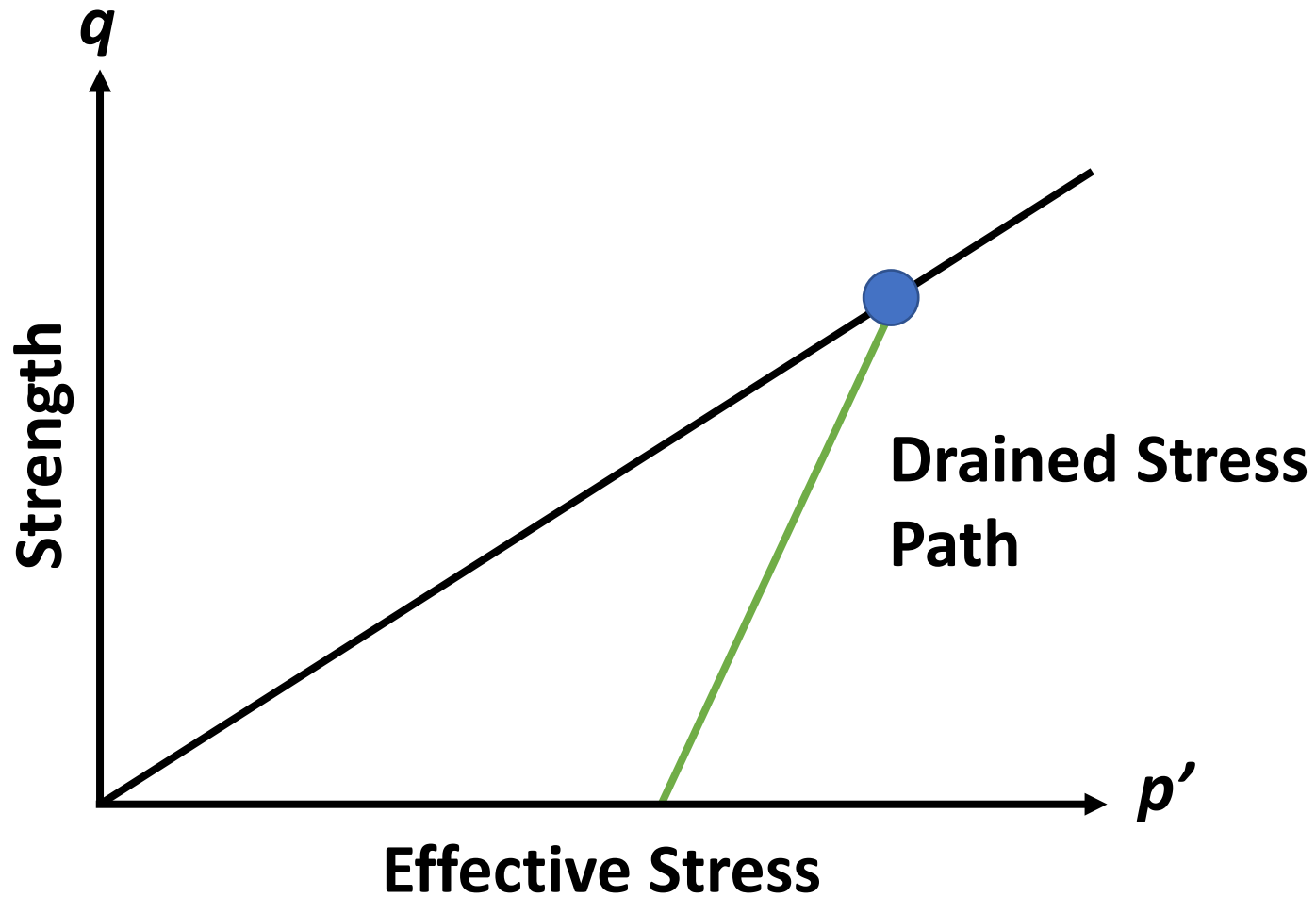
Drained vs. Undrained Behavior

- Pertains to rate of shear/loading and material permeability
- If excess pore-water pressures develop → undrained
- If no excess pore-water pressures develop → drained

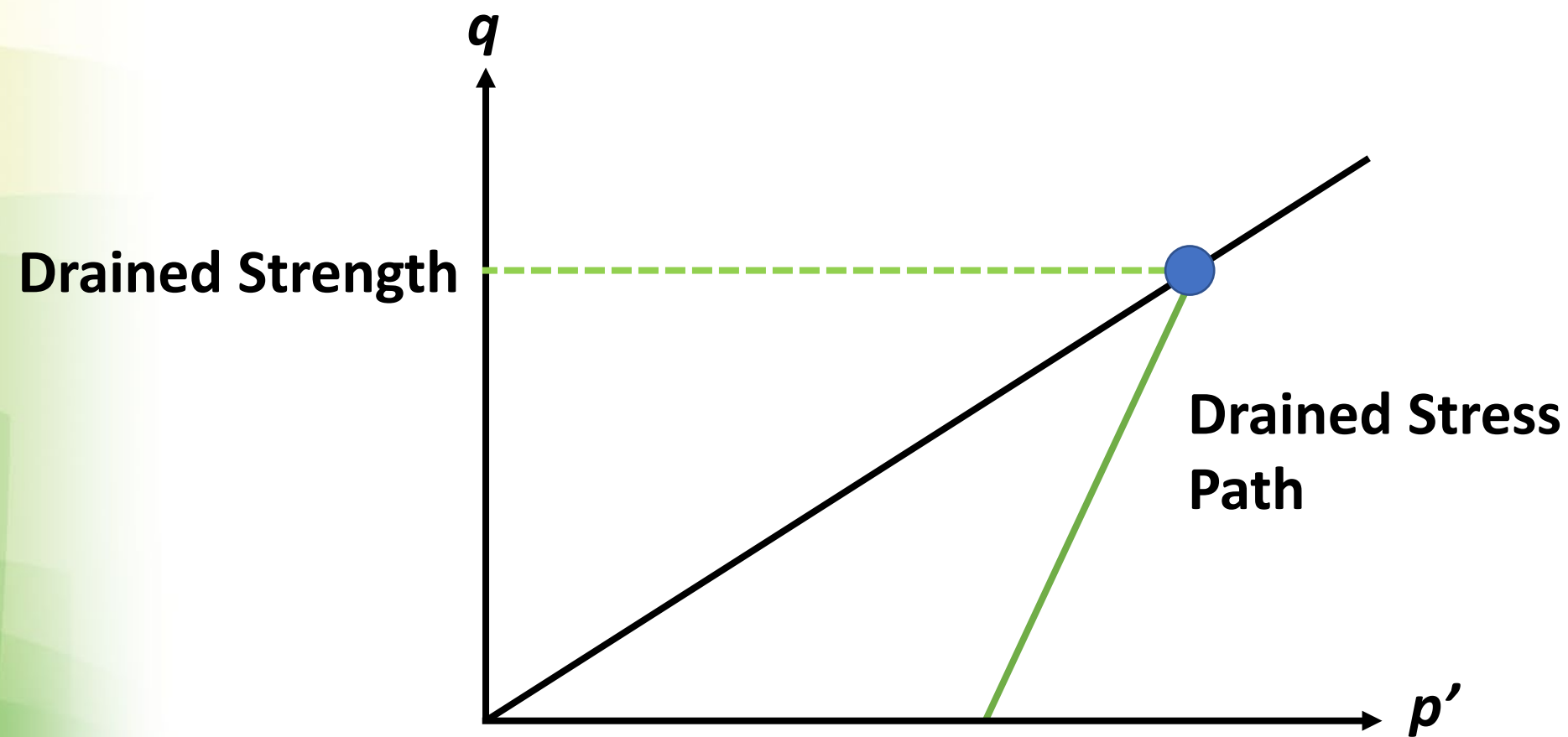
Drained vs. Undrained Behavior (cont.)



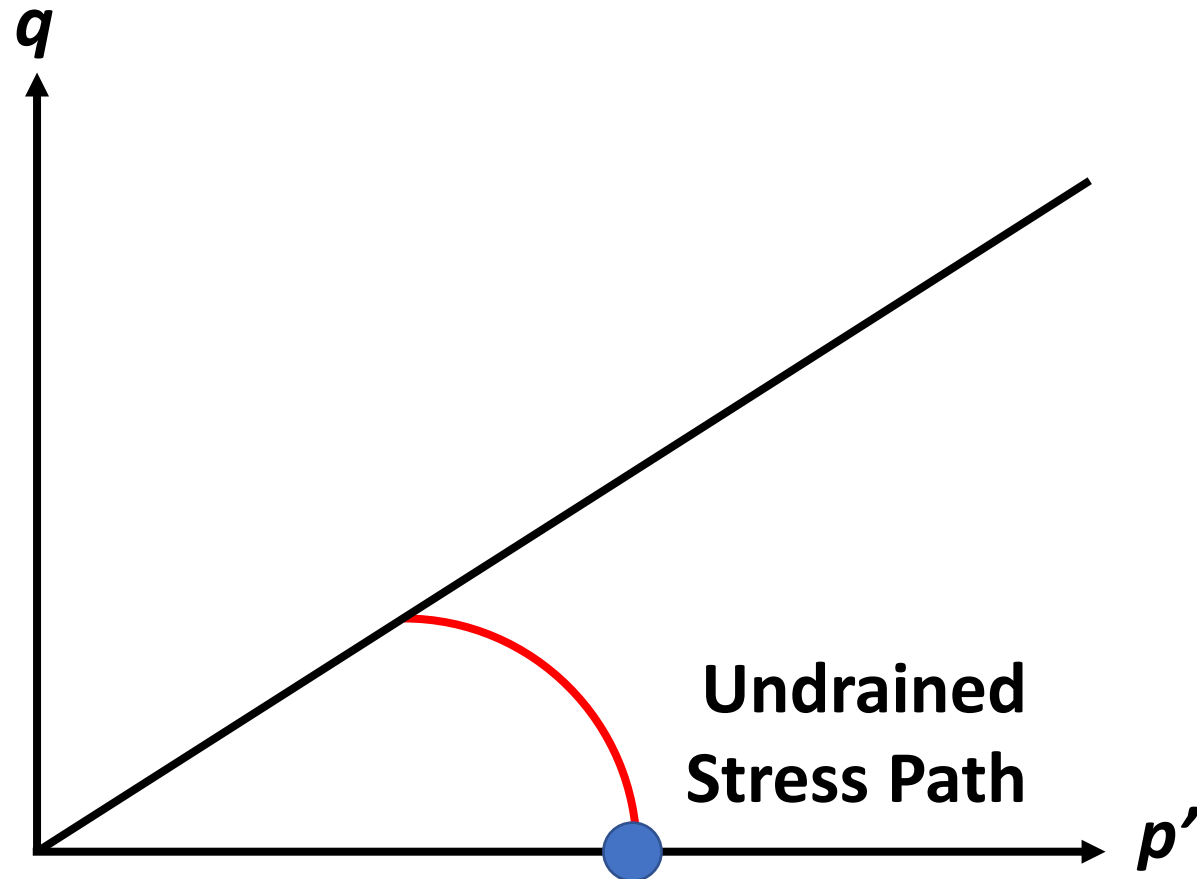
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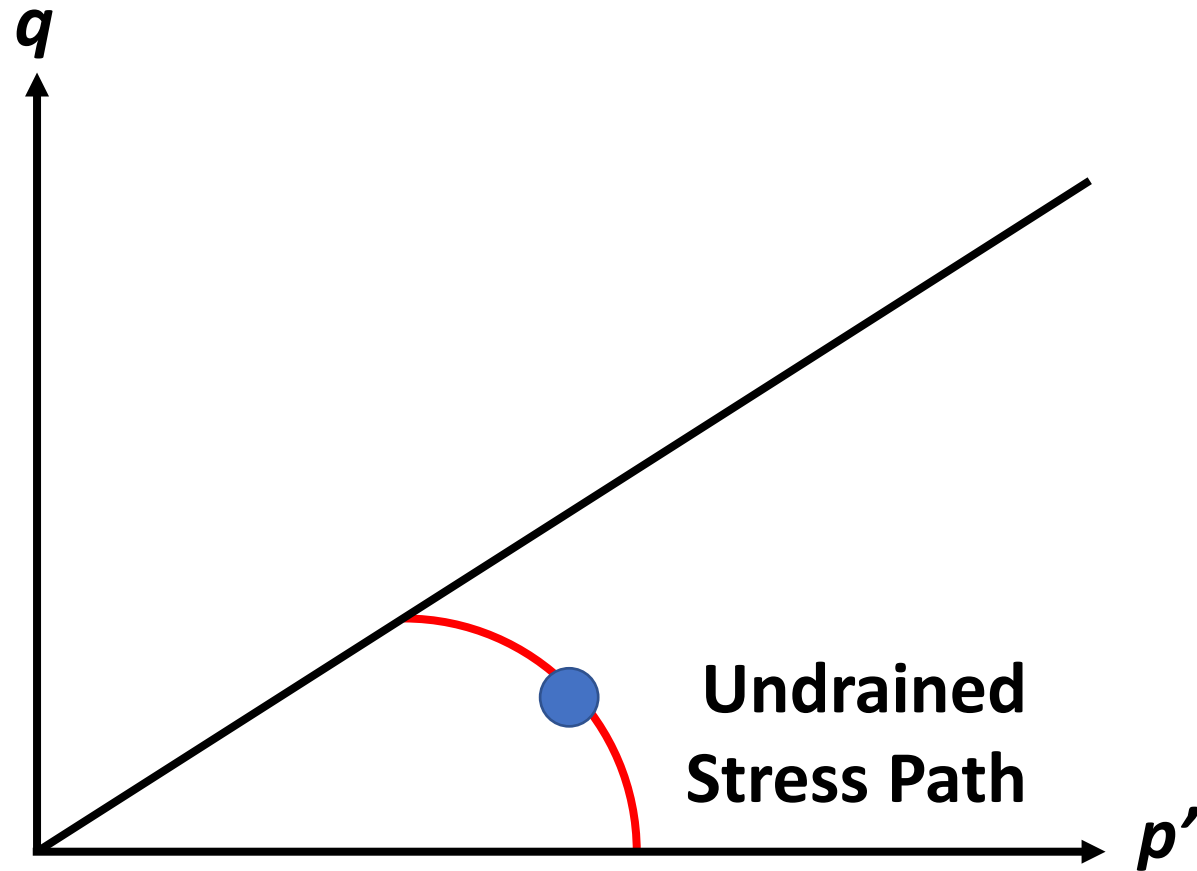
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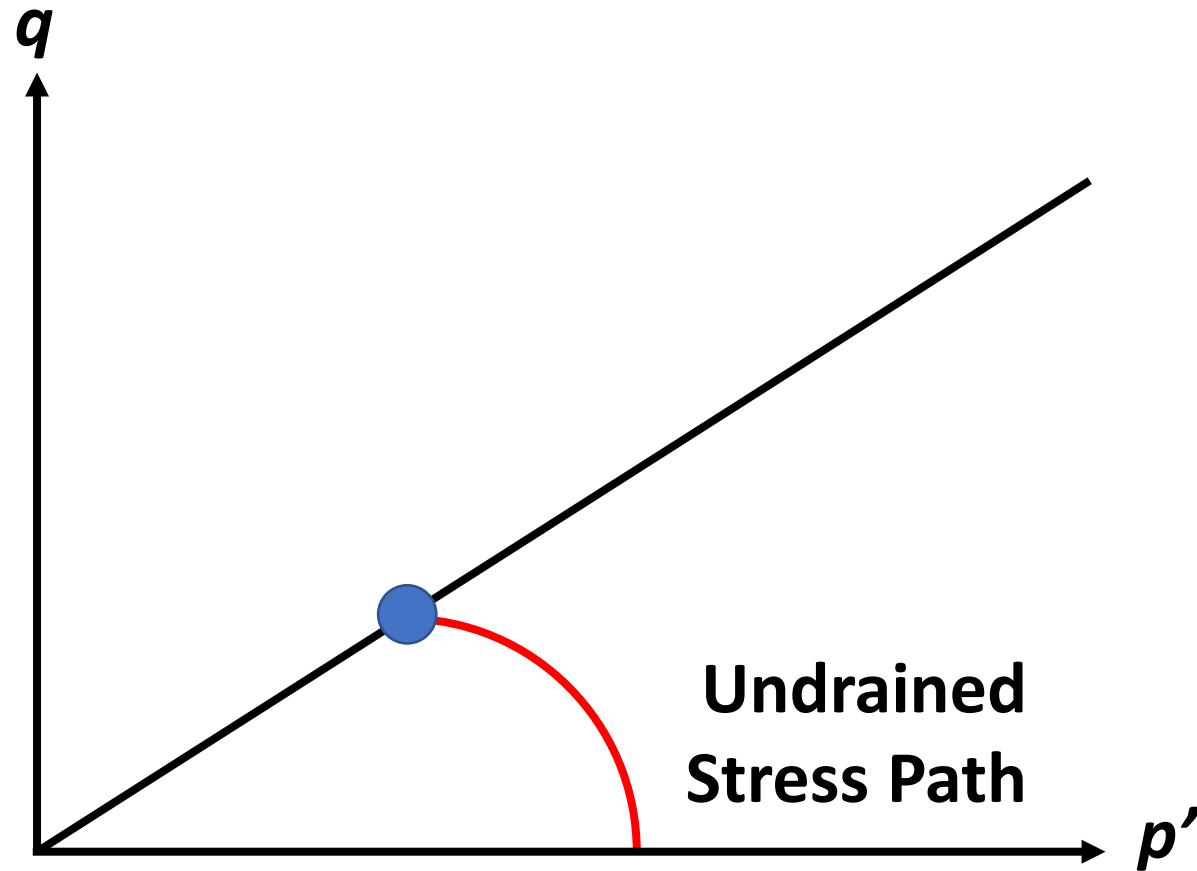
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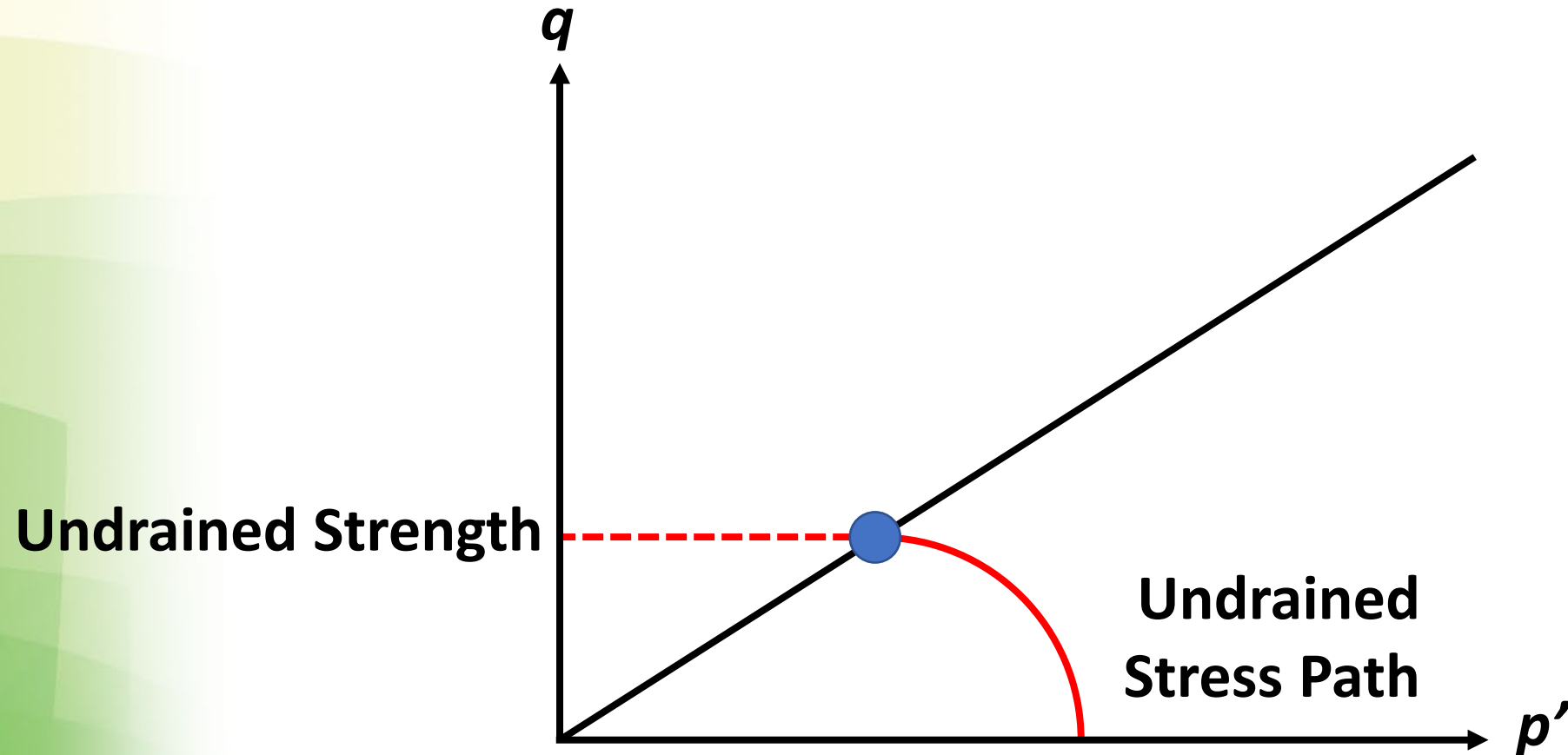
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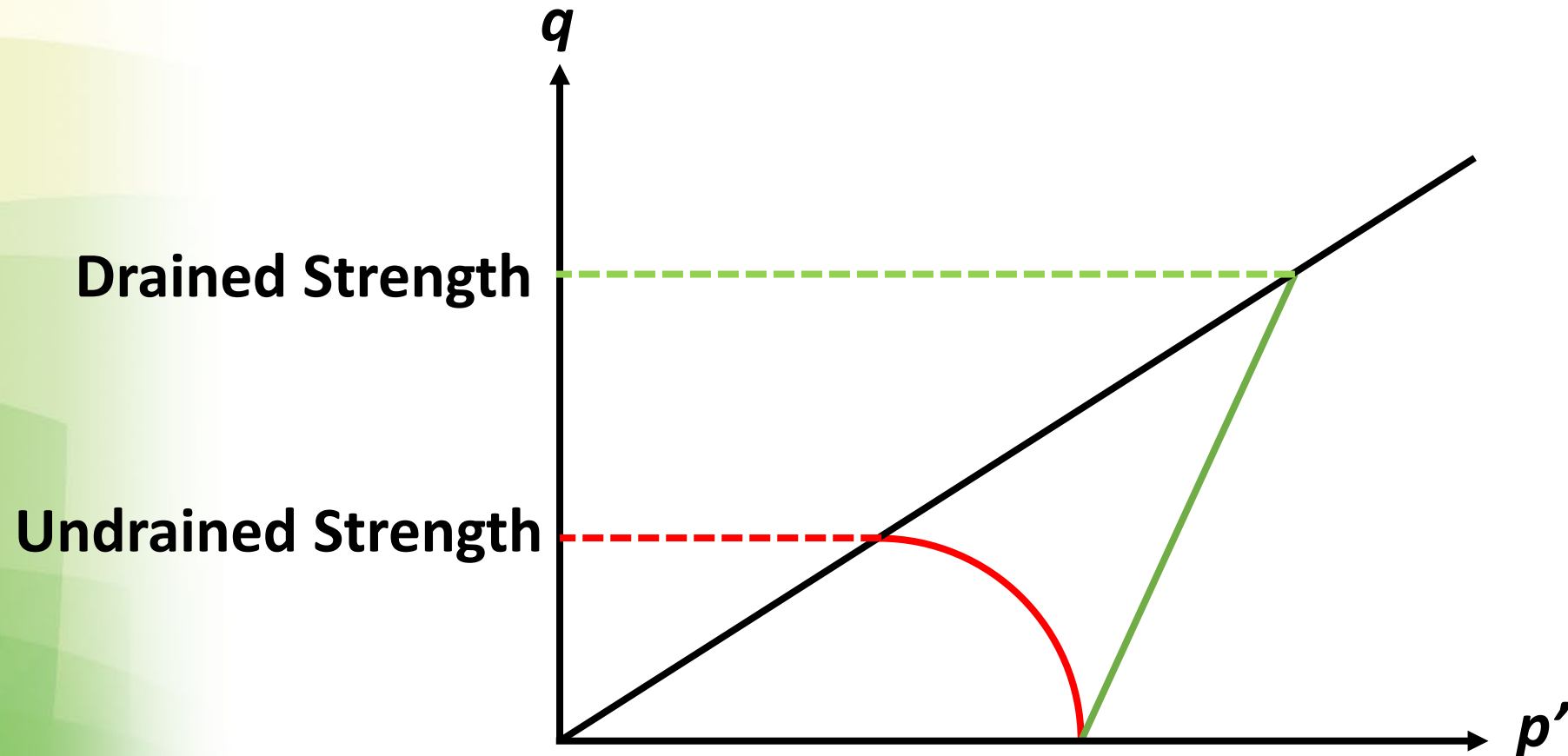
Drained vs. Undrained Behavior (cont.)



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Drained vs. Undrained Behavior (cont.)



Drained vs. Undrained Behavior (cont.)

Characteristic	OC	NC
Shear Behavior	Dilative	Contractive
Excess Pore-Water Pressure	Negative, then 0	Positive
Typical Failure Condition	Drained	Undrained
Relative Strength	Higher	Lower

Drained vs. Undrained Behavior (cont.)

Characteristic	OC	NC
Shear Behavior	Dilative	Contractive
Excess Pore-Water Pressure	Negative, then 0	Positive
Typical Failure Condition	Drained	Undrained
Relative Strength	Higher	Lower



If an OC clay becomes NC and we don't account for the change in behavior, we may be unconservative

Stress History And Normalized Soil Engineering Properties (SHANSEP)

10664

JULY 1974

GT7

JOURNAL OF THE GEOTECHNICAL ENGINEERING DIVISION

NEW DESIGN PROCEDURE FOR STABILITY OF SOFT CLAYS

By Charles C. Ladd,¹ M. ASCE and Roger Foott,² A. M. ASCE

SHANSEP (cont.)

- Inter-relates stress history and strength

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- Allows for determination of strength at varying OCRs

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- Accounts for disturbance through over-consolidation during laboratory testing

SHANSEP (cont.)

- Inter-relates stress history and strength
- Allows for determination of strength at varying OCRs
- Accounts for change in behavior when material goes from OC to NC
- Accounts for disturbance through over-consolidation during laboratory testing
- Is implemented using laboratory consolidation and strength tests

SHANSEP (cont.)

$$\frac{s_u}{\sigma'_{vc}} = \left(\frac{s_u}{\sigma'_{vc}} \right)_{NC} (OCR)^m$$

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Assessment Methodology

Collect Samples

Perform Laboratory Consolidation
Testing

Calculate OCR

Perform Laboratory Strength
Testing

Develop Stress History-Strength
Relationship

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Collect Samples

Increasing Sample Quality



3-in Thin Wall Tubes

5-in Thin Wall Tubes

5-in Thin Wall Tubes with Piston Sampler

Block Sample



Collect Samples

Increasing Sample Quality



3-in Thin Wall Tubes

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5-in Thin Wall Tubes with Piston Sampler

Block Sample



Assessment Methodology

Collect Samples

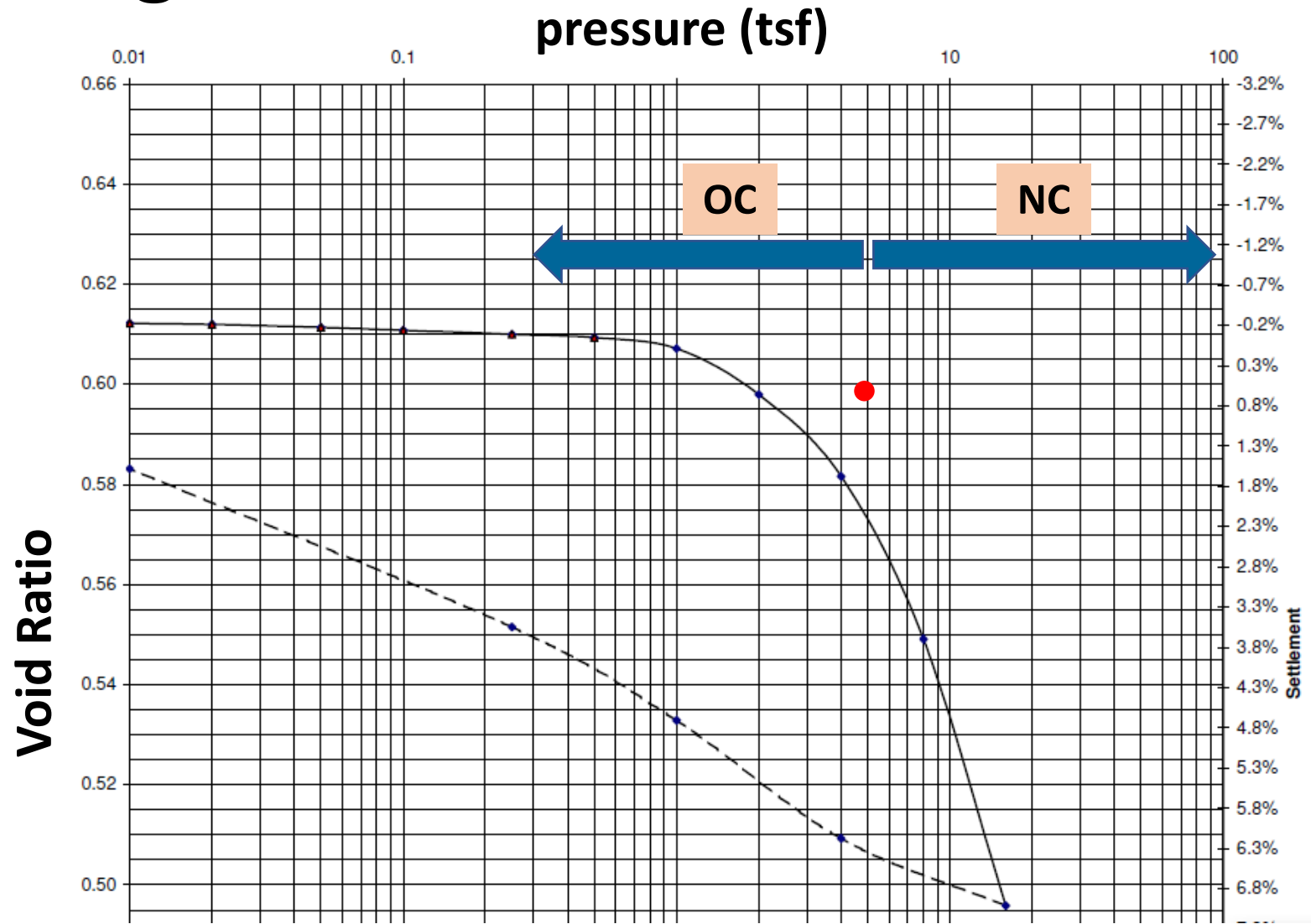
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Perform Laboratory Consolidation Testing



Assessment Methodology

Collect Samples

Perform Laboratory Consolidation
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Calculate OCR

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Develop Stress History-Strength
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Perform Laboratory Consolidation Testing

Max past pressure (from lab)

↓

$$\text{Over Consolidation Ratio} = OCR = \frac{\sigma'_p}{\sigma'_{v0}}$$

↑

Current pressure (in field)

Assessment Methodology

Collect Samples

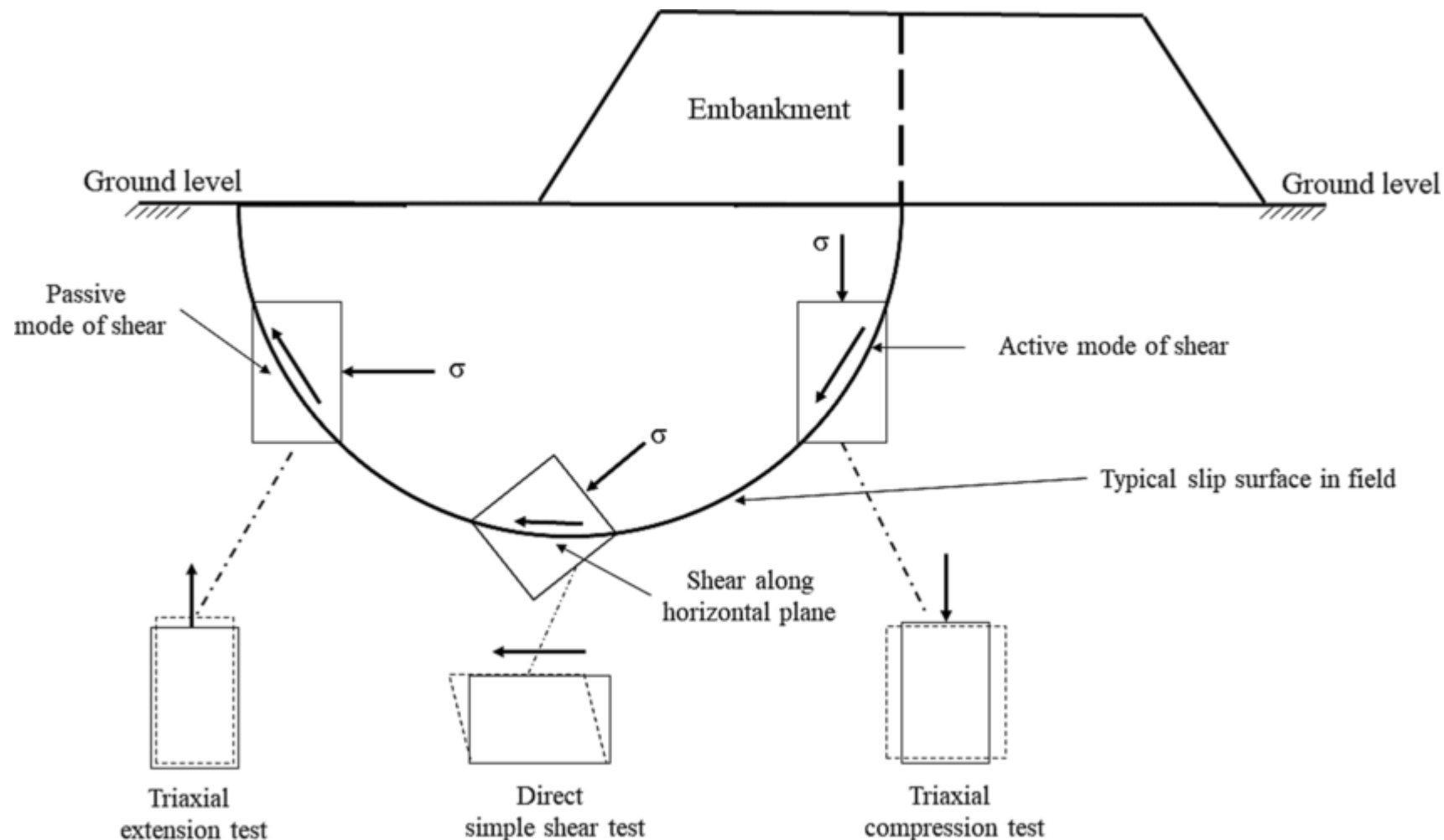
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Perform Laboratory Strength Testing



Assessment Methodology

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Develop Stress History-Strength Relationship

$$\frac{s_u}{\sigma'_{vc}} = \left(\frac{s_u}{\sigma'_{vc}} \right)_{NC} (OCR)^m$$

Develop Stress History-Strength Relationship (cont.)

$$\frac{s_u}{\sigma'_{vc}} = \left(\frac{s_u}{\sigma'_{vc}} \right)_{NC} (OCR)^m$$



Undrained shear
strength ratio at
 $OCR = 1$

Develop Stress History-Strength Relationship (cont.)

$$\frac{s_u}{\sigma'_{vc}} = \left(\frac{s_u}{\sigma'_{vc}} \right)_{NC} (OCR)^m \quad \leftarrow \text{Curve fit from lab testing}$$

↑
Undrained shear
strength ratio at
OCR = 1

Develop Stress History-Strength Relationship (cont.)

Undrained shear
strength ratio at
given OCR

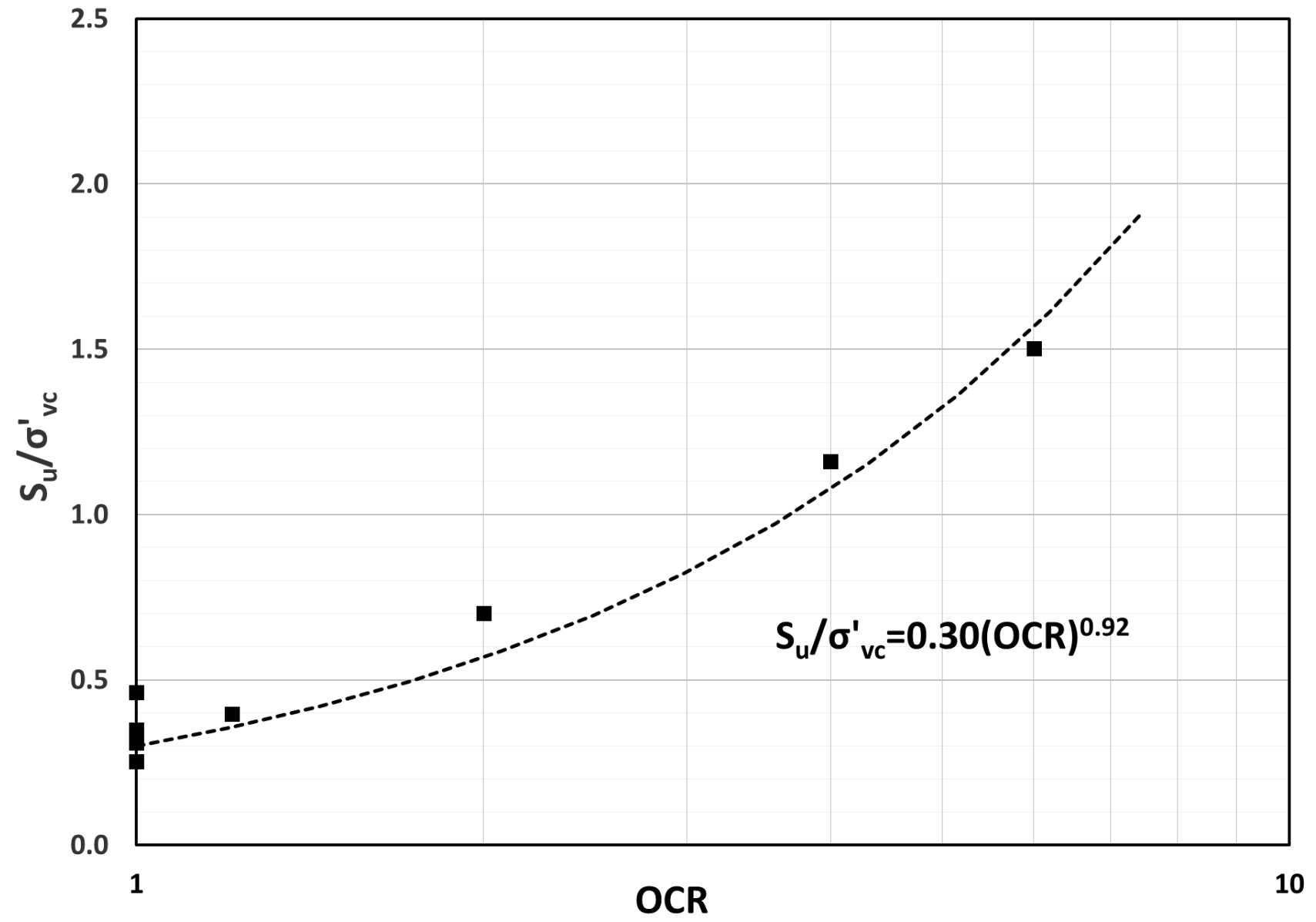
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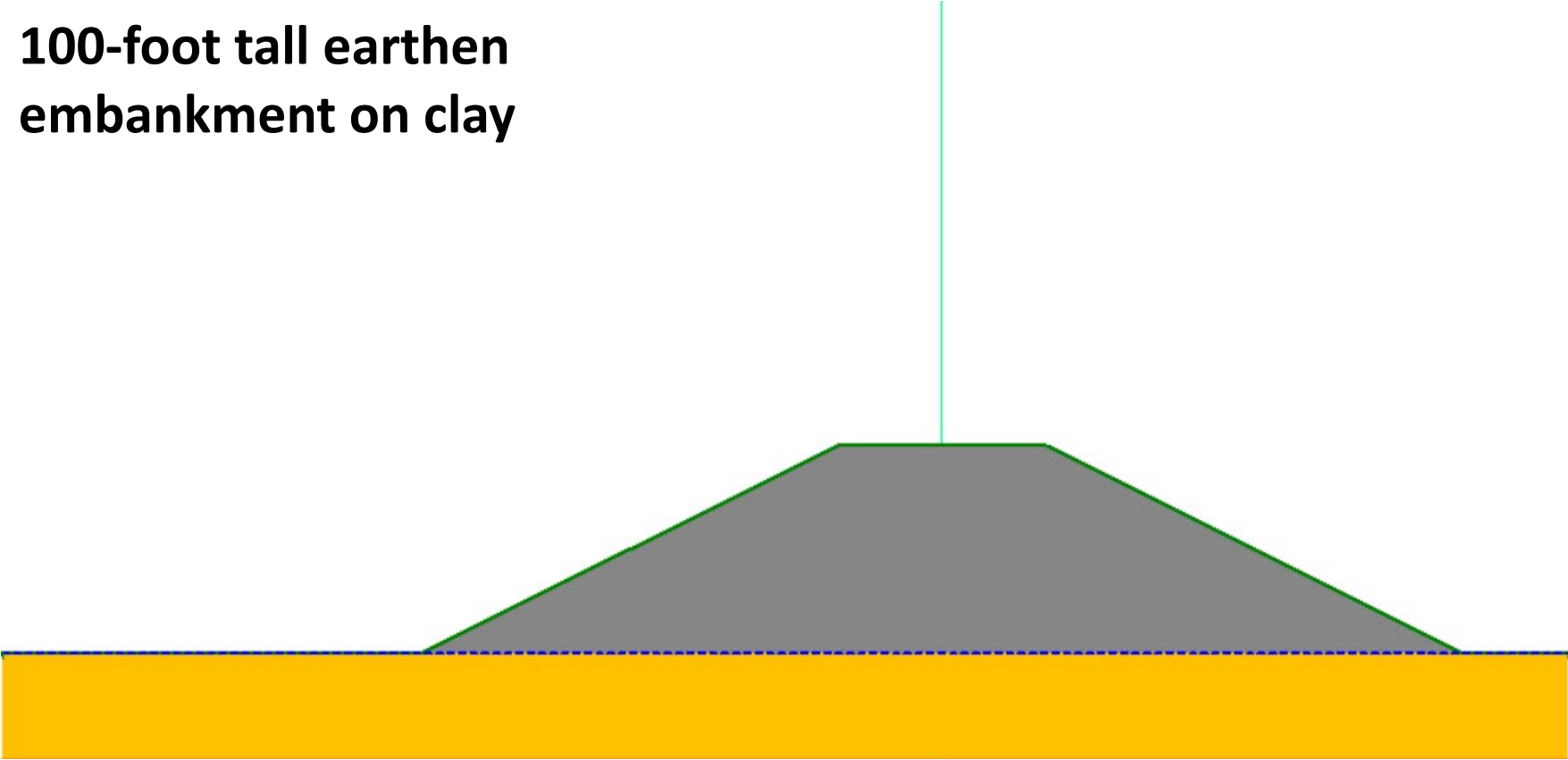


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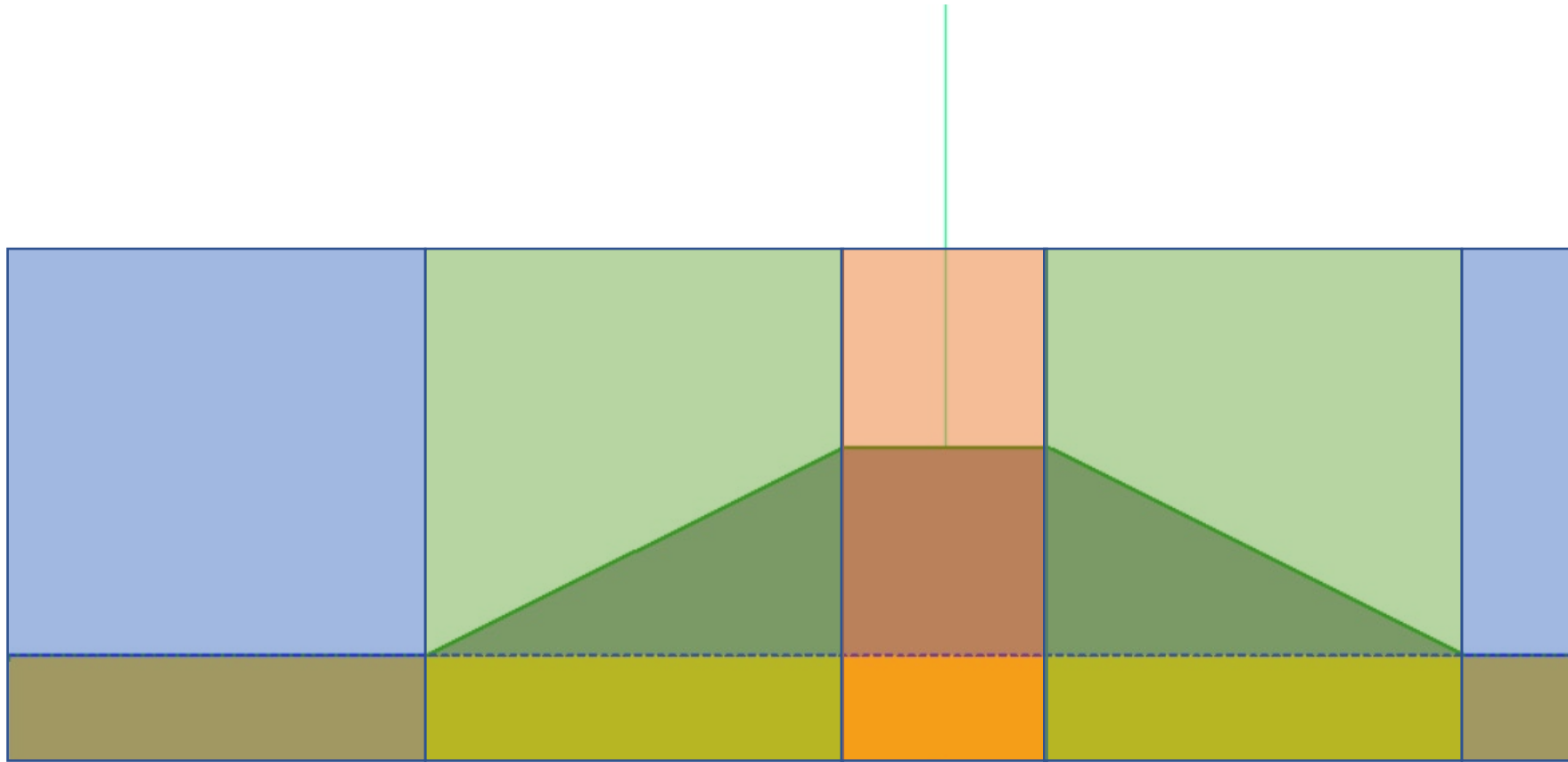
Hypothetical Example

100-foot tall earthen
embankment on clay



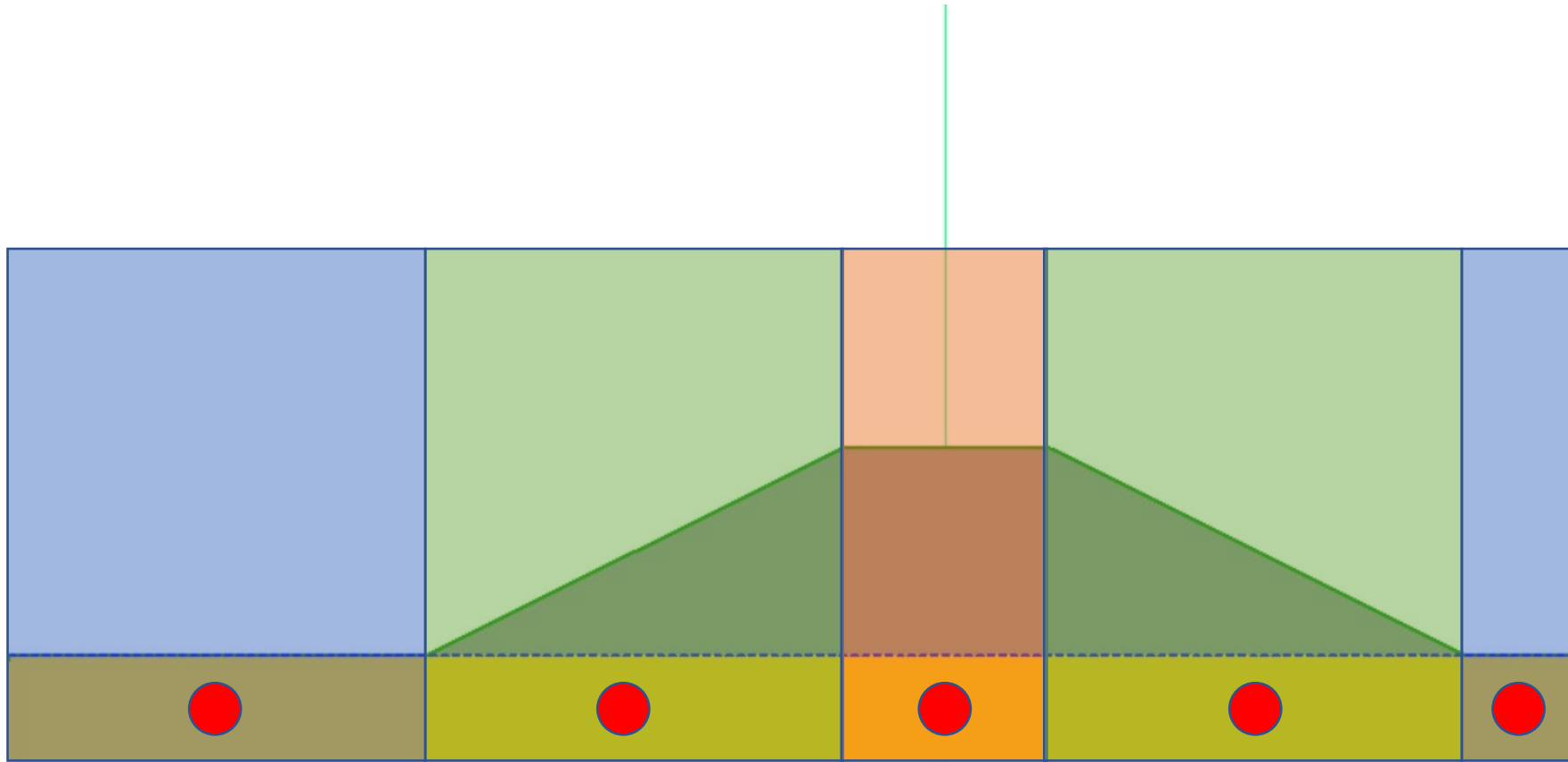
Hypothetical Example (cont.)

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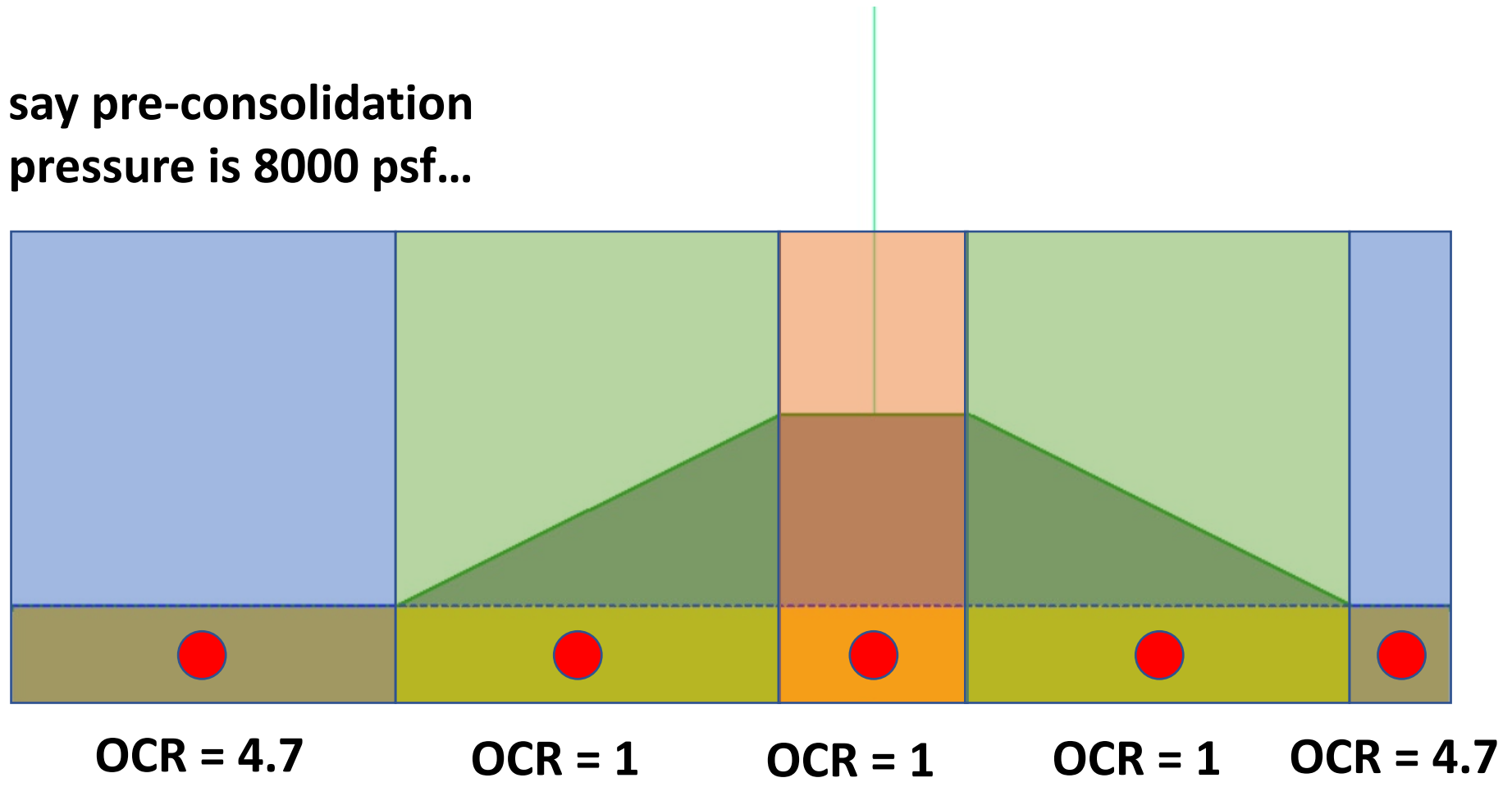
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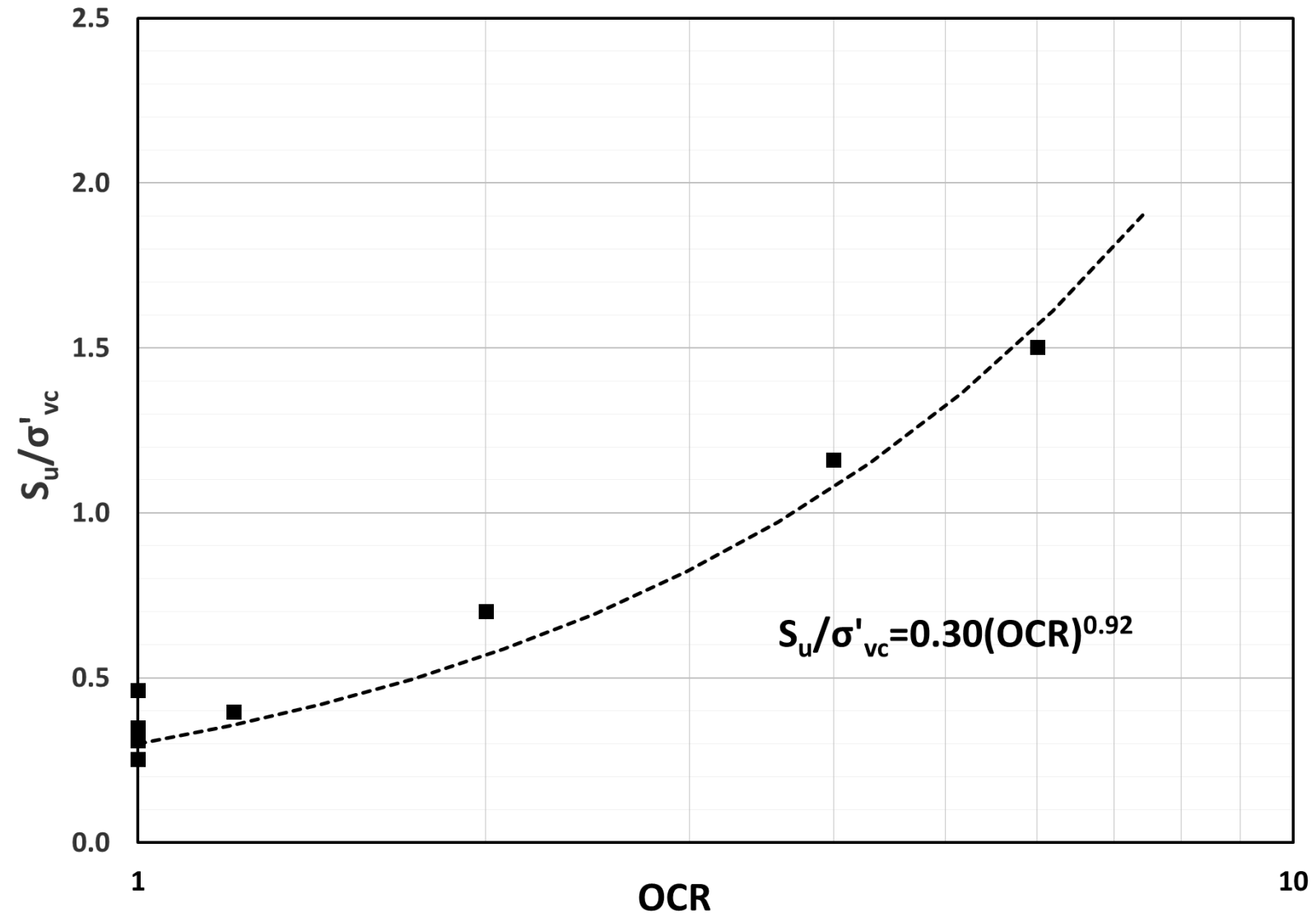
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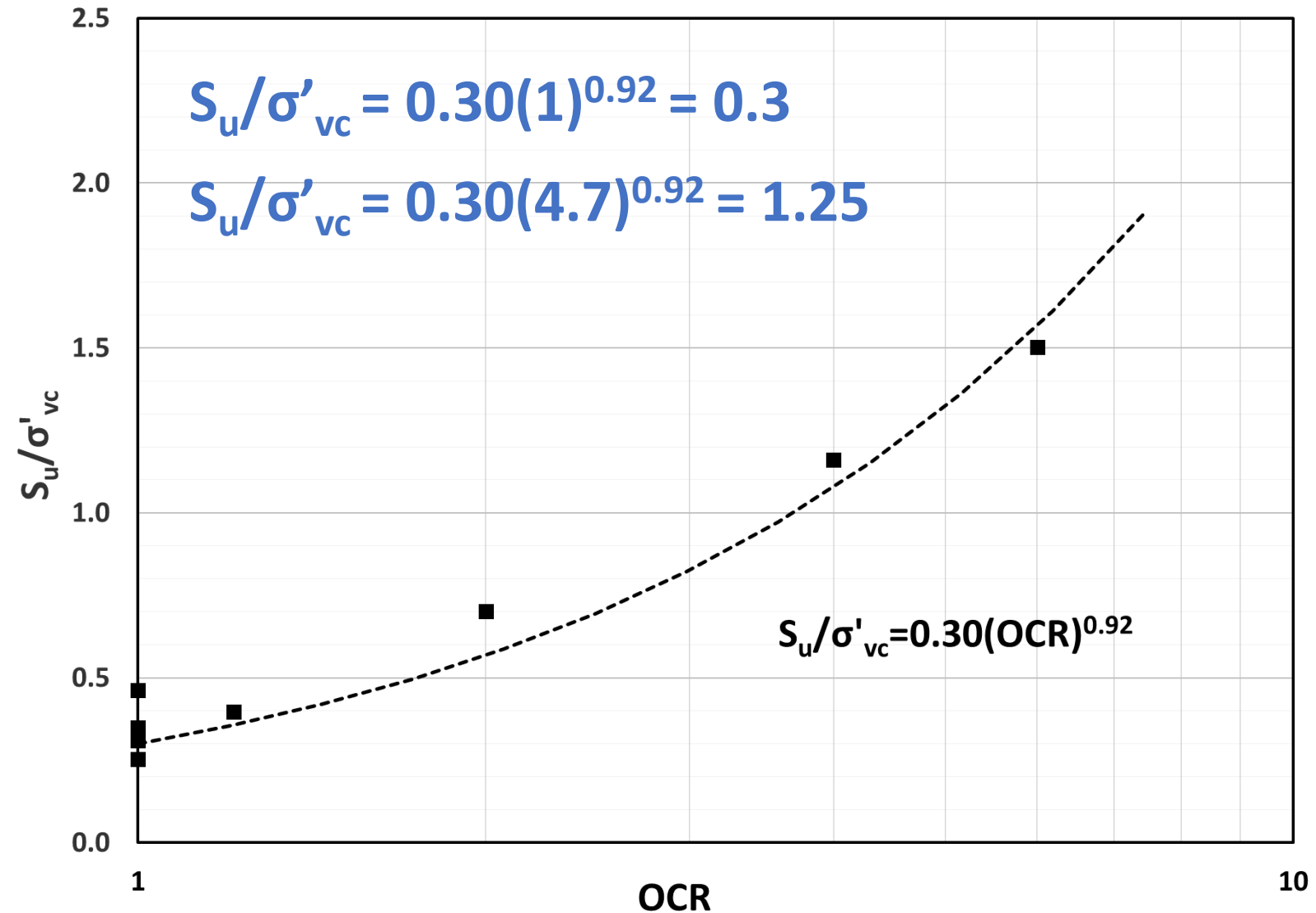
say pre-consolidation
pressure is 8000 psf...



Hypothetical Example (cont.)

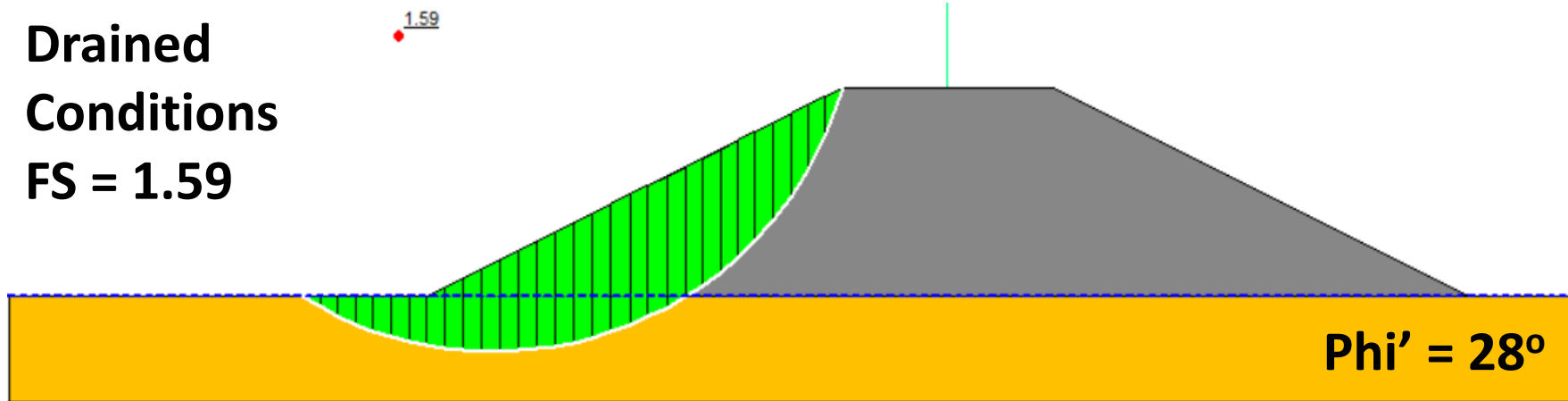


Hypothetical Example (cont.)



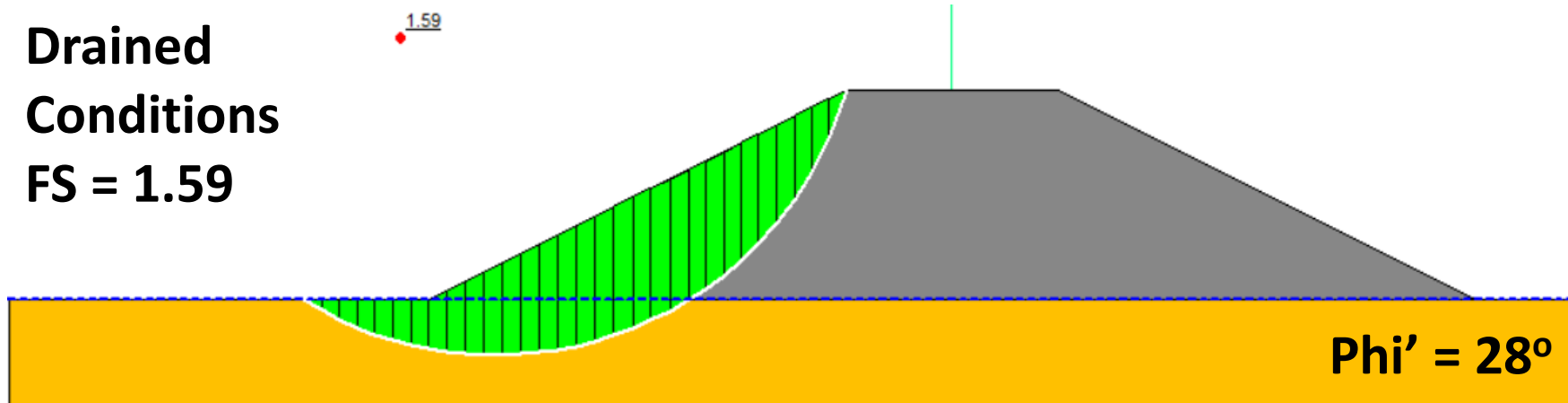
Hypothetical Example (cont.)

Drained
Conditions
 $FS = 1.59$

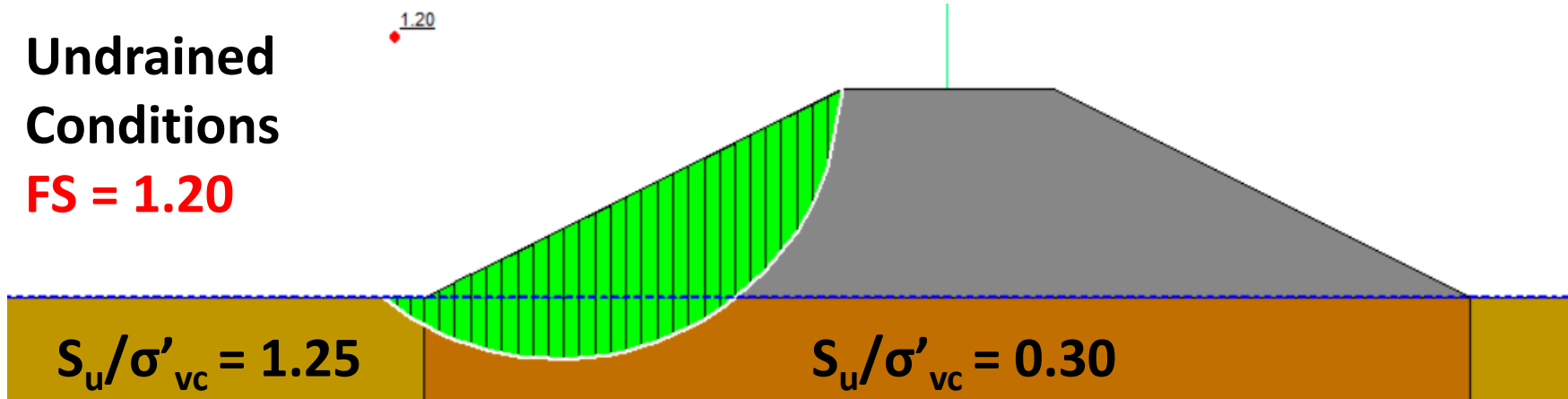


Hypothetical Example (cont.)

Drained
Conditions
FS = 1.59



Undrained
Conditions
FS = 1.20



Summary

- Much of northern Minnesota is underlain by stiff clay due to glaciation

Summary (cont.)

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- As clay is loaded, its behavior changes

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- Much of northern Minnesota is underlain by stiff clay due to glaciation
- As clay is loaded, its behavior changes
- Stress history influences strength
- Failure to account for change in strength due to loading may result in unconservative designs
- SHANSEP is a convenient approach for inter-relating strength and stress history

Thank You



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resourceful. naturally.

